

## **USAGE OF NATURAL AND RECYCLED COARSE AGGREGATES OF SELF CURING CONCRETE**

**P Lavanya**, Assistant Professor, Department Of Civil Engineering Sree Chaitanya College of Engineering, Karimnagar

**N Shiny Grace** Student Department Of Civil Engineering Sree Chaitanya College of Engineering, Karimnagar

### **ABSTRACT**

The problem of decreased cement hydration due to incorrect curing and therefore inferior qualities of concrete is resolved by providing a self-curing concrete that absorbs water from the atmosphere to achieve greater cement hydration in concrete.

In the current study, dosages of polyethylene glycol (PEG) with a molecular weight of 6000 (PEG 6000), a self-curing agent, are added to mixing water at intervals of 0.5 to 2% by weight of cement. The following was the proposed experimental program. A total of 120 cubes, 120 cylinders, and 120 prisms were cast using four distinct mixes (Mix 30 and Mix 40), various aggregates (recycled coarse aggregate and normal coarse aggregate), and varying dosages (0%, 0.5%, 1%, and 2%) of the self-curing agent PEG-6000. Studies comparing the self-curing of regular coarse aggregate and recycled coarse aggregate were conducted. Water retentivity, compressive strength, split tensile strength, and flexural strength for 7 and 28 days were compared between conventionally cured and self-cured concrete in comparative studies. Self-cured concrete has qualities that are, at the very least, on par with conventionally cured concrete, if not better. The strength comparison investigations were conducted using varying dosages of distinct coarse aggregate. The findings suggest that self-curing concrete exhibits higher strength in comparison to traditional curing concrete. Normal coarse aggregate concrete reaches its maximum strength at a dose of 2% SCA, while recycled coarse aggregate concrete reaches its maximum strength at 1% SCA before declining. At 28 days, it was discovered that the compressive strength of concrete with regular coarse aggregate instead of recycled coarse aggregate was 7.5% higher. Self-curing concrete has a 3% higher compressive strength than conventionally cured concrete, which dries out in 28 days.

Similarly, at the same 28-day testing, it was discovered that the split tensile strength with regular coarse aggregate was 7% higher than with recycled coarse aggregate. Once more, after 28 days of testing, it was discovered that the split tensile strength was 3.1% greater than the concrete with traditional curing. When comparing 28-day recycled coarse aggregate concrete to natural coarse aggregate concrete, the former has a 5% higher flexural strength. When self-curing concrete is compared to conventional curing concrete over a 28-day period, the flexural strength increases by 2%. When the percentage of SCA grows, the maximum weight loss in the concrete happens at 0% of SCA.

### **Keywords:**

PEG 6000, OPC, recycled coarse aggregate, self-curing agent, and curing.

## **I. INTRODUCTION**

### **1.1) Curing**

Balancing the rate and volume of transpiration from concrete through cement hydration is identified as curing. It could be either after it has been set in situ (or when making concrete products), allowing enough time for the cement to hydrate. If concrete is to reach its full durability and strength potential, curing must be done for a suitable amount of time because cement hydration takes several days or weeks instead of hours. Due to its effect on how soon cement hydrates, temperature regulation may

also be incorporated into the definition of curing. The required qualities of the concrete, the intended application, and the environmental conditions, such as the air's temperature and relative humidity, may all affect the curing period.

### **Conventional Curing Methods**

Following are some general categories for concrete curing techniques:

- i. Reduce the escape of moisture from the concrete by, for instance, by overlaying it with an outer layer that is largely impermeable.
- ii. Oven curing and Steam curing.
- iii. Continue misting the concrete's exposed surface in order to avoid moisture loss.
- iv. Ponding or water spraying the surface.

### **Challenges with the Traditional Curing**

- i. It is not possible to maintain the surface moisture for the vertical element as it is for flat surfaces.
- ii. In areas where there is a water shortage.
- iii. In locations where physical curing is not feasible.

### **1.2. Self Curing**

Curing entails maintaining the early stages of concrete at an appropriate moisture level so that it can develop the required properties. Good healing, however, is not necessarily practical in many circumstances. Many researchers looked into the viability of producing self-curing concrete (SCC). Thus, the requirement to create self-curing chemicals attracted a number of researchers. The idea behind self-curing compounds is to boost concrete's ability to retain water by reducing the amount of water that evaporates from it as compared to regular concrete. It was discovered that water soluble polymers might be employed in concrete as self-curing agents (SCA). Concrete's durability and performance are enhanced by curing because it has a significant impact on how the material develops its pore structure and microstructure. The idea behind self-curing compounds is to boost concrete's ability to retain water by reducing the amount of water that evaporates from it as compared to regular concrete. Since each cubic metre of concrete takes around 3 cubic metres of water for production, the majority of which is used for curing, the use of self-curing admixtures is crucial.

### **1.3. Definition of self-curing**

It has long been believed that concrete cures "from the exterior to the inside" by establishing circumstances that prevent water from escaping beneath the surface. On the other hand, "internal curing" enables curing "from the inside to outside" by using the inside reservoirs that were created, such as saturated wood fibers, and lightweight fine aggregates. It's also common to refer to "internal curing" as "self-curing."

### **1.4. Potential Materials for self curing**

The following materials, such as LWA 19mm Coarse, Lightweight Aggregate, Super- absorbent Polymers, LWS Sand, SRA, and Wood powder, can serve as internal water reservoirs.

### **1.5. Chemicals to Achieve Self-curing**

If specific water-soluble additives are used during the mixing process to stop water evaporation from and inside the set concrete, concrete might start to "cure" on its own. In a normal cement matrix, the substances ought to be capable to improve the holding capacity of water and reduce solution evaporation. Some substances, such as polyethylene glycol (PEG), polyvalent alcohol, xylitol, sorbitol, poly-acrylic acid, glycerin phytosterols, stearyl alcohol, etc are hydrophilic in nature.

### **1.6 CLASSIFICATION OF AGGREGATES**

The following categories are used in this report for organisational purposes.

#### **1.6.1 Natural Aggregate**

Construction aggregates include, for example, crushed river gravel, sand, crushed rock, and gravel which are extracted from natural resources like gravel and sand.

#### **1.6.2 Manufactured Aggregate**

A few examples of produced aggregates are expanded shale, clays, slate, polystyrene aggregate (PSA), fly ash aggregate, foamed blast furnace slag (FBS), manufactured sand, and by-products of manufacturing processes.

#### **1.6.3 Recycled Aggregate**

Reclaimed Aggregate (RA), Reclaimed Concrete and Masonry (RCM), Reclaimed Asphalt Aggregate (RAA), Glass Cullet, Reclaimed Asphalt Pavement (RAP), Utilised Foundry Sand, and Scrap Tyres are a few kinds of aggregates made from materials that had been utilised in construction.

#### **1.6.4 Reused By-product**

Aggregates made from industrial waste include, for example, Mine Tailings, Fly Ash (FA), Crusher Fines, Furnace Bottom Ash (FBA), Organic Materials, Incinerator Bottom Ash (IBA), Electric Arc Furnace Slag (EAF), Coal Washer Reject (CWR), Steel Furnace Slag (BOS), and Granulated BF Slag (GBS).

#### **1.7 Sources of Recycled Aggregate**

In the past, landfills have used Portland concrete aggregate from demolition projects. However, Portland concrete aggregate can now be employed as a new kind of building material. According to OPC concrete Recycling, crushed Portland concrete pavements and building materials are the principal source of recycled aggregates.

#### **1.9 The Use of Recycled Aggregate In Concrete**

An alternate coarse aggregate that is often mixed with ordinary coarse aggregate for use in novel concrete is crushed aggregate from either hardened residual concrete or demolition concrete. If not adequately regulated and monitored, the use of 100% recycled coarse aggregate in concrete is likely to have a detrimental impact on the majority of the characteristics of concrete, including compressive strength (CS), creep, elastic modulus (EM), etc especially for higher-strength concrete.



#### **1.10 Need of Present Work**

Curing entails maintaining the early stages of concrete at an appropriate moisture level so that it can develop the required properties. Good healing, however, is not necessarily practical in many circumstances. Many researchers looked into the viability of producing self-curing concrete. Thus, the requirement to create self-curing chemicals attracted a number of researchers. In contrast to ordinary concrete, the idea behind self-curing chemicals is to increase the concrete's ability to retain water by reducing the amount of water that evaporates from it.

Self-curing concrete is an option to enhance cement hydration in concrete. It resolves the problem that insufficient or improper curing decreases cement hydration, resulting in undesired concrete properties.

#### **1.11 Mechanism of self-curing**

The variation in chemical potentials among the liquid and vapour phases causes continual evaporation of water from a surface that is exposed, which is the basis for the self-curing process. The provided polymers work largely by creating hydrogen bonds with water molecules and reducing the chemical potential of the molecules to slowly decrease the rate at which water escapes from the outermost layer.

## **II. LITERATURE REVIEW**

### **Roland et al., (2002) [1]**

The objective of the research was to produce self curing concrete by using hydrophilic chemicals like polyethylene glycol and paraffin wax. Many experiments have done on ordinary concrete like compressive strength at different days of curing and also at different proportions of PEG and wax. The investigation was done using three internal curing compositions and is as follows:-

Curing material	Curing membrane	Internal curing Composition 1	Internal curing Composition 2	Internal curing Composition 3
Base material	Solvent borne Resin with dye	Wax Emulsion High MW Polyethylene oxide and Water	Water, paraffin Wax & Polyethylene glycol	Water based polyether's

### A.S. EL- DIEB (2007) [2]

The aim of the study was to compare SCA to traditional concrete curing in order to determine the SCA potential for water holding, degree of hydration, and moisture transfer. This study used polyethylene glycol, a water-soluble polymeric glycol, as the SCA. By weight of cement, the SCA dosage was 0.02%. For all SCC mixtures, the dose was maintained at the same level.

The study's objective was to examine concrete that had variable cement content at various water to cement ratios, including self-curing, air-curing concrete, and conventional to compare the results of various tests. According to the weight loss over time, it was discovered that SCA infused concrete mixes have higher water retention than ordinary concrete mixes. Compared to ordinary concrete (OC), SCC saw less self-desiccation under sealed settings. In comparison to traditional concrete, SCC exhibited better hydration over time under drying conditions. SCC has a lower water transport than standard air-cured concrete. For SCC, water permeability, and sorptivity values dropped over time.

### Wen et al., (2008) [3]

The study's goal was to determine how the use of high-performance SCA affected the strength properties of self-compacted concrete in compared to OC. In this investigation, polyvalent alcohol and polyacrylic acid (PAA) were used as the SCA. The nature of these two substances is highly hydrophilic. SCA dosages ranged between 1% and 2% by weight of cement. Tests on CS and water retention were conducted at relative humidity levels of 50%, 67.5%, and 85%. According to the weight loss over time, it was discovered that SCA infused concrete mixes have higher after retention than OC mixes. Compared to OC, SCC saw less self-desiccation under sealed settings. In comparison to traditional concrete, SCC exhibited well hydration over time under drying conditions. Self-curing concrete has a lower water transport than standard air-cured concrete. For SCC, water permeability and water sorptivity values dropped over time, indicating a decreasing proportion of permeable pores as a result of the cement's ongoing hydration. With this technology, the compressive strength (CS) of self-compacting concrete was greatly increased, often surpassing that of conventional moist curing. This proves that the SCC of this invention does not require a lengthy moist curing process, saving money and ensuring greater quality and a higher CS.

### Collepardi [5]

This study work's goal was to create a drying shrinkage-free concrete (SFC), even in non-wet curing circumstances, by combining the usage of:

- a polycarboxylate-based water-reducing additive that minimises cement and water consumption while boosting aggregate content;
- a unique polycarboxylate (PA/SRA) with a shrinkage-reducing admixture (SRA) made from polyethylene glycol that can lower the surface tension of liquid water entering through the capillary pores.;

At 30 minutes after mixing, 5 concrete mixtures with w/c of 0.50 and slump levels of 170- 200 mm were produced. Firstly, there is a decrease in the volume of cement paste and a rise in the amount of aggregate because of the typical decreased water content generated by the polycarboxylate superplasticizer at a certain w/c. Both play a significant role in the decrease of drying shrinkage.

Secondly, the SRA set in the system of polyethylene glycol escapes as a result of the alkaline environment of the aqueous solution brought on by the hydration of the cement. This further reduces drying shrinkage, which is linked to a reduction in water's surface tension, and this lowers capillary pressure.

### Dhir et al., [4]

This study presents the findings of several durability tests on self-curing concrete. Preliminary surface absorption, potential variation (PV) chloride dispersion, depth of carbonation, half-cell corrosion potential, and determination of freeze/thaw durability were the experiments that were conducted. During the program, 3 mixes were used: one with simply OPC as a binder, 1 with a 40% GGBS cement substitution, and 1 with PFA as a 30% cement substitute. There were 2 dosages applied: 0.005M and 0.100M.

### III. EXPERIMENTAL PROGRAMME

The following was the proposed framework of the experimental program:

A 120 cubes, cylinders, and prisms were cast using 4 distinct mixes such as A1, A2, B1, and B2) and various curing circumstances (indoor, traditional) with varying dosages of the self- curing ingredient PEG-6000 (0%, 0.5%, 1%, and 2%). To govern the properties of fresh concrete, the compaction factor test was performed on all mixtures. CS tests were conducted at 7 and 28 days after curing, and to examine the cubes' ability to retain water, they were weighed every 3 days starting from the casting date. The digital scale that is utilised has a 5 gm accuracy. A graph of strength versus the % of the self-curing agent and a graph of water retentivity versus the average weight loss throughout the number of curing days are both shown.

The highest self-curing agent dose in this inquiry is limited to 2%, and the smallest dosage is established in accordance with the literature accessible [2].

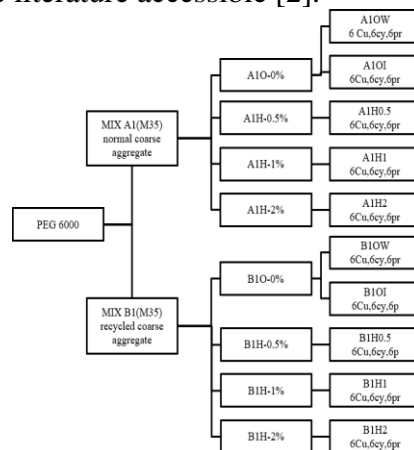
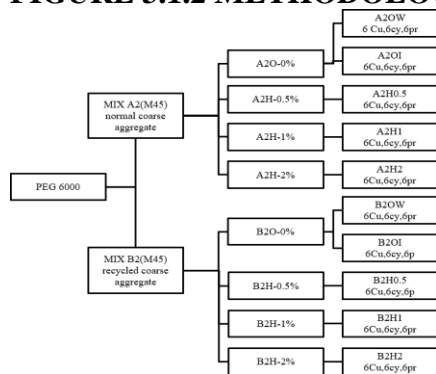


Fig:3.1.1FLOW CHART OF EXPERIMENTAL PROGRAMME FOR CONCRETE

### FIGURE 3.1.2 METHODOLOGY



### SPECIMEN NOMENCLATURE

MIX A- standard coarse aggregate(A1- M35,A2- M45 grades) MIX B- recycled coarse aggregate (B1- M35,B2-M45 grades) OPC-Ordinary Portland cement

Higher Molecular Weight -PEG 6000

I- Inside curing

W-Wet/Conventional Curing

S C A-Self-Curing Agent

### 3.2) Materials Used

#### Cement

Table 3.2.1 Physical properties of cement

Initial setting time	76 min
Final setting time	218 min
Specific gravity	3.13

Employed was fine aggregate that complies with IS 383-1970's Falls in zone-II. The neighbouring river course provided the fine aggregate that was used. Each sieve's sand retention was put into a different bag, which was then stacked separately for use and conformed as zone-II sand. Tables 3.2.2 respectively, indicate the physical characteristics of fine aggregate.

Specific gravity	2.60
Bulk density	1.37gm/cc
Fineness modulus	2.80

The coarse aggregate of 12 to 20mm nominal size coarse material is used and satisfying the IS-383 guidelines. The quarry-purchased coarse aggregate was passed through each of the sieves up to 4.75mm According to tables 3.2.3, respectively, each fraction's physical characteristics.

Specific gravity	2.7
Bulk density	1.60gm/cc
Fineness modulus	7.4

The recycled coarse material utilised is 20 mm nominally sized and comes from a lab crushing plant. In this experiment, 20mm well-graded aggregate in accordance with IS-383 is used. All the upto 4.75 mm were used to separate the recycled coarse material that was purchased from the lab. According to Tables 3.2.4 each fraction's physical characteristics displayed.

Bulk density	1327 kg/m <sup>3</sup>
Fineness modulus	7.38
Specific gravity	2.4

Injurious pollutants like alkalis, oil, acids, and other substances shouldn't be present in the water used to make concrete. Concrete was created and allowed to cure in the laboratory using regular drinkable water. It was determined that the water quality met IS: 456 -2000 standards.

The standard formula for PEG is  $H(OCH_2CH_2)_nOH$ , where  $n$  is the average number of repeated oxyethylene groups, commonly ranging from 4 to around 180. PEG is a condensation polymer of ethylene oxide and water.. The PEG is followed by a numerical suffix that describes the average molecular weights. PEG seems to have a common property called water soluble. Table 3.2.8 displays the PEG6000's specs. Additionally, it is soluble in a variety of organic solvents, including hydrocarbons that are aromatic (not aliphatic). Identical chemical and physical qualities are provided by the large variety of chain lengths for the right application selections indirectly or directly.

$$\text{HO}-\text{CH}_2-\text{CH}_2-\text{O}-\text{CH}_2-\text{CH}_2-\text{OH}$$



S.No.	Specification	PEG 6000
1	Mol Wt.	5500-6500
	Color, Bohn	10 max
3	Hydroxyl Value	16-23 (mg KOH/g)
4	Ph	5 - 7
5	Moisture	0.5% max
6	Appearance	white flake
7	Dioxane	1ppm max
8	Specific Gravity	1.08 - 1.09

Table 3.2.8 Specifications of PEG 6000

## IV. RESULTS & DISCUSSION

### 4.1. STUDIES ON CONCRETE

#### 4.1.1. COMPACTION FACTOR TEST

To determine workability and calculate the compaction factor, the compaction factor test is carried out. Table 4.1 presents the test findings. Figure 4.1 displays a plot of the compaction factor versus various PEG 6000 dosages. The observations from the Compaction Factor Test are listed below. It is evident from the specimens with PEG 6000 of Mix A that the 0.5% dosage of the SCA has a lower compaction factor than the 1% and 2% dosages. The compaction factor for specimens containing PEG 6000 of Mix B is higher as contrasted with the previous concentrations (1% and 2%). It is also evident that Mix B has a higher compaction factor than Mix A by 1% and 2%. Another observation is that Mix A's compaction factor increases as the percentage of PEG 6000 increases. But in Mix B, it goes from 0.5% to 1% before being reduced.

### 4.2 Water Retentivity Test

#### 4.2.1. Mix A1 Water Retentivity Test

Weighing the samples with a digital scale with an accuracy of 5 gms for a period of 28 days at regular intervals allowed us to study concrete containing high molecular weight PEG that had been subjected to indoor curing. Table 4.2 has a record of the outcomes. Table 4.3 displays the analysis of the findings of the weight loss of each specimen. In fig.4.2, the weight decrease is displayed. The observations below pertain to concrete's water retentivity. It is evident that the 0% dosage of the SCA causes greater weight loss than the 0.5%, 1%, and 2% dosages. Additionally, it has been noted that the 2% dosage of the SCA results in less weight loss than the 0%,.5%, and 1% dosages.

Table 3.3.1 Materials Required for Mix A1

SL.NO.	Nomenclature of Mix	No. of cubes			Cement (kg)	FA (kg)	CA (kg)	Water (Lit)	PEG 6000 (gm)
		Cubes	cylinders	Prisms					
1	A1OW	6	6	6	44.25	65.4	89.6	17.7	0
2	A1OI	6	6	6	44.25	65.4	89.6	17.7	0
3	A1H0.5	6	6	6	44.25	65.4	89.6	17.7	221
4	A1H1	6	6	6	44.25	65.4	89.6	17.7	442
5	A1H2	6	6	6	44.25	65.4	89.6	17.7	884

Table 3.3.2 Materials Required for Mix A2

SL.NO.	Nomenclature of Mix	No. of cubes			Cement (kg)	FA (kg)	CA (kg)	Water (Lit)	PEG 6000 (gm)
		cubes	cylinders	prisms					
1	A2OW	6	6	6	49.6	53.2	93.1	17.8	0
2	A2OI	6	6	6	49.6	53.2	93.1	17.8	0
3	A2H0.5	6	6	6	49.6	53.2	93.1	17.8	248
4	A2H1	6	6	6	49.6	53.2	93.1	17.8	496
5	A2H2	6	6	6	49.6	53.2	93.1	17.8	992

Table 3.3.3 Materials Required for Mix B1

SL.NO.	Nomenclature of Mix	No. of cubes			Cement (kg)	FA (kg)	CA (kg)	Water (Lit)	PEG 6000 (gm)
		Cubes	cylinders	prisms					
1	B1OW	6	6	6	44.25	65.4	91.6	17.2	0
2	B1OI	6	6	6	44.25	65.4	91.6	17.2	0
3	B1H0.5	6	6	6	44.25	65.4	91.6	17.2	221
4	B1H1	6	6	6	44.25	65.4	91.6	17.2	442
5	B1H2	6	6	6	44.25	65.4	91.6	17.2	884

TABLE 4.1 Compaction Factor for different percentages of PEG 6000

Percentage Dosage of PEG	Compacting Factor	
	AH	BH
0.5	0.98	0.934
1	0.988	0.976
2	0.996	0.956

Table 4.2 Water Retentivity Test for Mix A1 for different percentages of PEG 6000

Table 4.2.1 Average weight of cubes at different ages(kg)							
DESIGNATION	0	3	7	10	14	20	28
A10W	8.502	8.485	8.497	8.50	8.517	8.52	8.5
A10I	8.433	8.436	8.427	8.450	8.445	8.400	8.399
A1H 0.5	8.665	8.693	8.600	8.628	8.59	8.56	8.54
A1H 1	8.559	8.558	8.541	8.560	8.55	8.54	8.53
A1H 2	8.599	8.558	8.562	8.593	8.572	8.56	8.52

Table 4.2.2 Avg weight of cylinders at different ages(kg)							
DESIGNATION	0	3	7	10	14	20	28
A10W	13.07	13.02	13.02	13.05	13.02	13.01	13.00
A10I	13.01	13.05	13.03	13.04	13.01	12.9	12.78
A1H 0.5	13.02	13.0	13.15	13.14	13.12	13.00	12.83
A1H 1	13.09	13.01	13.00	13.10	13.01	13.02	12.99
A1H 2	13.20	13.11	13.15	13.09	13.12	13.09	13.1

Table 4.2.3 Avg weight of prisms at different ages(kg)							
DESIGNATION	0	3	7	10	14	20	28
A10W	12.550	12.5	12.597	12.586	12.517	12.59	12.65
A10I	12.433	12.336	12.327	12.390	12.395	12.391	12.2
A1H 0.5	12.636	12.493	12.620	12.628	12.597	12.543	12.545
A1H 1	12.541	12.538	12.511	12.560	12.525	12.523	12.500
A1H 2	12.579	12.658	12.542	12.593	12.560	12.559	12.555

Tables 4.3 Average Weight Loss for Mix A1 for different percentage of PEG 6000

Table 4.3.1 Avg Weight Loss for Mix A1(cubes) for different percentage of PEG 6000

Designation	Curing Period, Days							Weight loss Ratio
	0	3	7	10	14	20	28	
A10W	0	-0.022	-0.025	-0.33	-0.034	-0.036	-0.037	
A10I	0	0.058	0.057	0.067	0.069	0.077	0.080	1
A1H 0.5	0	0.019	0.024	0.027	0.029	0.035	0.035	0.75
A1H1	0	0.023	0.018	0.20	0.022	0.025	0.025	0.61
A1H2	0	0.018	0.019	0.020	0.024	0.024	0.025	0.52

Table 4.3.2 Avg Weight Loss for Mix A1(cylinders) for different percentage of PEG 6000

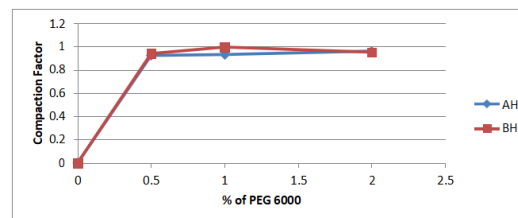


Fig 4.1 Compaction factor for different percentages of PEG 6000

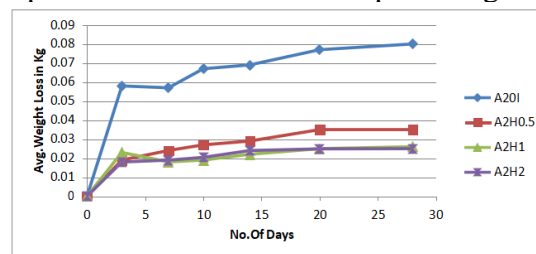


Fig:4.2.1.Avg Weight Loss for Mix A2 (CUBES) For the Different Dosages of PEG 6000

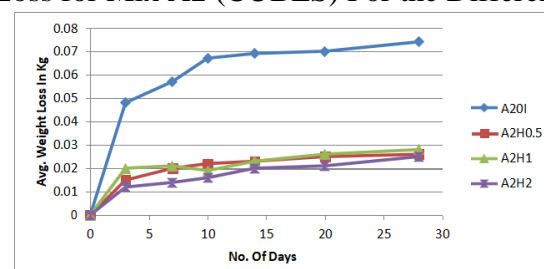




Fig:4.2.2 Avg Weight Loss for Mix A2 (CYLINDERS) For the Different Dosages of PEG 6000

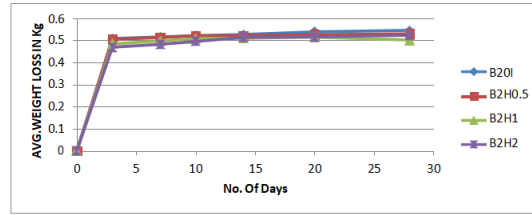


Fig 4.3.3AVG Weight loss for Mix A1 (prisms) for the different dosages of PEG6000

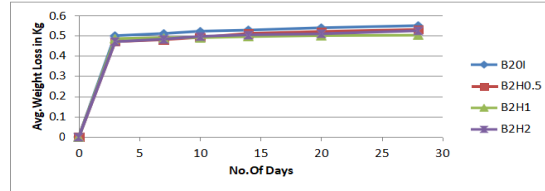
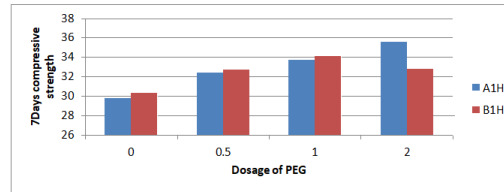
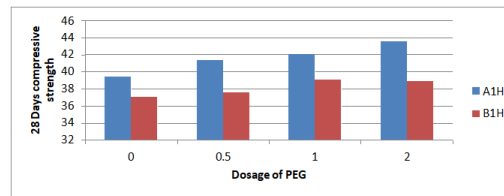


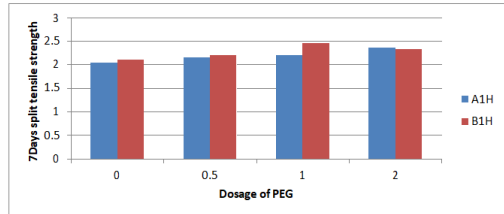
Fig: 4.4.1 Avg Weight Loss for MixB2 (CUBES) For the Different Dosages of PEG 6000



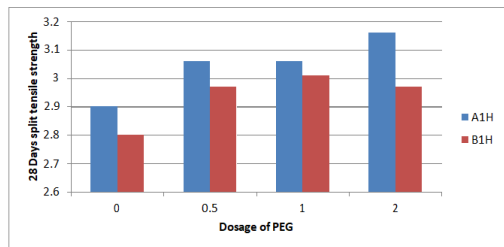
4.10.1 Variation of 7 days compressive strength with different dosages of PEG6000



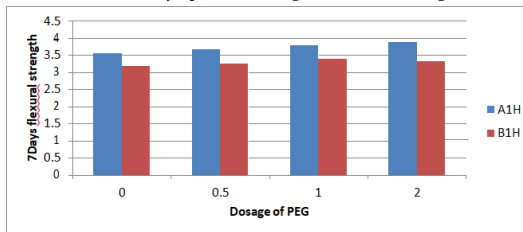
4.10.2 Variation of 28 days compressive strength with different dosages of PEG6000



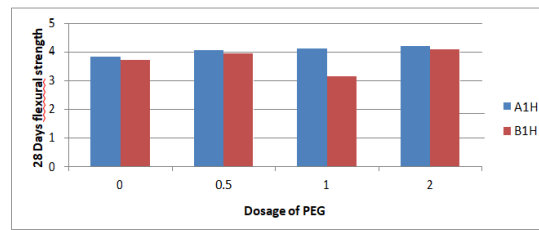
4.10.3 Variation of 7 days split tensile strength with different dosages of PEG6000



4.10.4 Variation of 28 days split tensile strength with different dosages of PEG6000



4.10.5 Variation of 7 days flexural strength with different dosages of PEG6000



4.10.1 Variation of 28 days flexural strength with different dosages of PEG6000

## V. CONCLUSIONS

- After analyzing the experimental program's outcomes, the following findings on SCA polyethylene glycol (PEG6000) and a comparison of different aggregates were made.
- 7.1. Findings
- As a result of using PEG6000
- • The workability of low w/c ratio concrete is significantly impacted by the use of higher molecular weight polyethylene glycol (PEG6000), and the water retention of low w/c ratio concrete is significantly impacted by the use of PEG6000.
- b) Findings from analyses comparing various coarse aggregate
- The efficacy of SCC is influenced by the w/c ratio and SCA % doses. Concrete mixes infused with SCA retain more water than OC mixtures. A greater CS is not necessary if there is less weight loss.
- c) Concrete is now more workable than it was with OC since normal coarse aggregate (PEG 6000) was used instead of OC.
- For concrete with a low water-to-cement ratio, the use of greater molecular weight polyethylene glycol (PEG6000) significantly affects water retention.

## REFERENCES

1. Wen-Chen Jau "Self-curing Concrete", United States Patent Application Publication, Pub. No: U.S. 2008/0072799 A1, Pub.date: Mar. 27,2008.
2. Roland Tak Yong Liang, Robert Keith Sun, "Compositions and Methods for Curing Concrete", Patent No.: US 6,468,344 B1, Date of Patent Oct. 22, 2002.
3. A.S.El-Dieb, "Self-curing Concrete: Water Retention, hydration and moisture transport", Construction and Building Materials Vol.21 (2007) 1282-1287.
4. A.S. El-Dieb, T.A. El-Maaddawy and A.A.M. Mahmoud, "Water-Soluble Polymers as Self-Curing Agent in Silica Fume Portland Cement Mixes", ACI Material Journal Vol.278 (2011) 1-18.
5. M. Collepardi, A. Borsoi, S. Collepardi, R. Troli and M. Valente , "Self-Curing, Shrinkage-Free Concrete ",ACI Material Journal SP 234-47 (2006) 755-764.
6. R.K. Dhir, P.C. Hewlett and T.D. Dyer," Durability of 'Self-Cure' Concrete",Cement and Concrete Research, Vol. 25. No. 6, 1153-1158,1995.
7. R.K. Dhir, P.C. Hewlett and T.D. Dyer, "An investigation into the feasibility of formulating ' self-cure' concrete", Materials and Structures Vol.27(1994), 606-615.
8. Raghavendra Y.B and Aswath M.U," Experimental investigation on concrete cured with various curing Methods-A Comparative Study", International Journal of Advanced Scientific Research and Technology, Issue: 2, Vol.3 (2012) 577-584.
9. Jagannadha Kumar M.V, Srikanth M, Rao K.Jagannadha,"Strength Characteristics of Self-curing Concrete", International Journal of Research in Engineering & Technology,Issue:1,Vol.1(2012) 51-57.
10. Ambily,P.S and Rajamane N.P, "Self-curing Concrete an Introduction", Structural Engineering Research Centre, CSIR, Chennai.